

# Response of P.E.B with various spans considering SSI

<sup>#1</sup>Deshpande Nikhil N, <sup>#2</sup>Prof. D. B.Kulkarni, <sup>#3</sup>Mr. Nitin Lale

<sup>1</sup>nikhildeshpande4058@gmail.com  
<sup>2</sup>dattatraya.kulkarni@ritindia.edu  
<sup>3</sup>technocracy.consultants@gmail.com



<sup>#1</sup>Research Scholar, Department of Civil Engineering, Rajarambapu Institute of Technology, Islampur, Maharashtra, India.

<sup>#2</sup>Professor, Department of Civil Engineering, Rajarambapu Institute of Technology, Islampur, Maharashtra, India.

<sup>#3</sup>Director, Technocracy Consultants, Manikbaug, Pune.

## ABSTRACT

In order to study structural behaviour of any structure, it is prominent to study the effects of soil structure interaction (SSI). In present study, an attempt have been made to study the influence of soil structure interaction on Pre Engineered Building (P.E.B) with various spans. Usually the structural behaviour is analysed assuming the fixed support conditions at the base of structure. In conventional method the foundation flexibility of soil mass is ignored which is likely to affect the structural response of building. The soil flexibility is integrated in the analysis of structure using Winkler's spring model approach. For analysis P.E.B with 15m, 21m, 24m and 30m spans is considered with equal bay spacing. Three different soil strata's i.e. hard, medium and soft are used for SSI study. The analysis is carried out in STAAD Pro.V8i software using response spectra of IS 1893-2002. The effect of SSI on various parameters like base shear, lateral displacement and column moment.etc are studied and discussed. To get real behaviour of superstructure the subgrade must be modelled adequately well. The study reveals that the SSI significantly affects the performance of the structure.

**KeyWords:** Soil Structure Interaction (SSI), Pre engineered building (P.E.B), Equivalent static method, etc.

## ARTICLE INFO

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## I. INTRODUCTION

An earthquake is a shaking of the ground caused by sudden rupture and movement of large tectonic plates. The Indian sub-continent has a history of devastating earthquakes. After Killari (1993), Jabalpur (1997), and Bhuj (2001) earthquake it is clear that no part of the country is free from the seismic hazard. The main reason for the high intensity of earthquake in India is because of the movement of Indian plate towards the Eurasian plate at the rate of 49mm per year approximately. Geographical statistics shows that the India has almost 54 percent of land vulnerable to seismic hazards. The advance countries like USA, Japan are already constructing the structures which can resist the earthquake of magnitude 7 and above. Unfortunately in India not much awareness has been created in society, about the importance of constructing earthquake resisting structures.

### 1.1 Soil structure interaction:

Most of the civil engineering structure involve some type of structural element having direct contact with ground. There are many circumstances in civil engineering for which

interaction between structure and ground has to be considered prominently. This encourages the interaction between structural engineers & geotechnical engineers. During to external lateral forces such as earthquake the structural displacement & ground displacement both are interdependent on each other. It is impossible to depart the correlation between structures & ground motion. It can be easily understand that the interaction between soil and structure can indeed affect the performance of the structure during earthquake particularly structure founded on relatively flexible soils.

Soil structure interaction is the general phenomena involved in the behaviour of structure which interacting with soil medium in response to the lateral loading imposed on the structure. The phenomena may be defined as "The process in which the response of soil influences the motion of respect to structure influence the response of the soil is termed as SSI". This phenomena deals with interaction between structure & sub soil

### 1.2 Need of soil structure interaction:

In India from last few decades there is significance increase in the infrastructural development of country. Since the structure are huge and heavy the effect like SSI are to be considered during the design procedure of such structures. The effect of SSI on structure is not considered in early stage of construction practices. But since last 3-4 decades it has achieved prominent importance to consider the SSI while designing the structure. The effect of SSI for light structure can be neglected but its effect on heavy structure like high rise buildings, bridges, tall chimneys, nuclear power plants (NPP), elevated highways becomes prominent for better performance of structure during earthquake. Many researchers have suggested different methods to study the effect of soil structure interaction during last few decades. Winkler's spring model (1867) represents the soil medium as of identical but mutually independent, closely spaced, discrete, linearly elastic springs. George G Gazetas (1991) has presented complete set of algebraic formulas and dimensionless charts for readily computing the dynamic stiffness of springs which represents the soil medium.[8]

### 1.3 Objective of study:

The primary objective of this work is to study the response of Pre engineered building by equivalent lateral force method using STAAD Pro.V8i software. The study has been carried out to investigate the influence of soil structure interaction with equal bay spacing in P.E.B, also to understand the influence of SSI on the performance of P.E.B with varying span considering three different soil strata's.

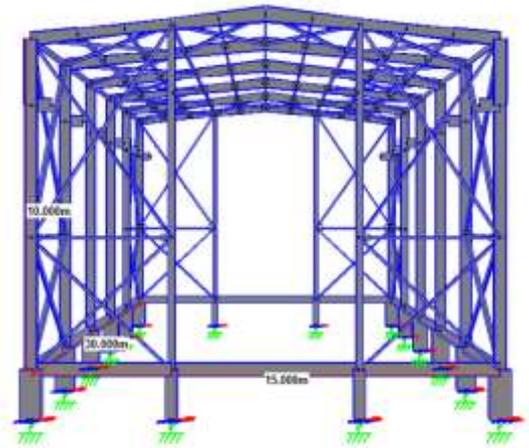
## II. STRUCTURAL MODELLING

For the analysis of work a Pre engineered building with 15m, 21m, 24m, 30m span is considered. The behaviour of this building is studied during earthquake excitation forces considering the soil structure interaction. The buildings are 11m high and length to width ratio is 2, height of crane girder is 8m. Building is symmetrical along both the X and Z-axis having equal bay spacing along Z-axis and along X-axis bay spacing is provided according to wind chart in Kirby building manual, each bay of 6m in Z-direction. Isolated footings are considered to be resting on three types of soil strata's namely, hard soil, medium soil, and soft soil.

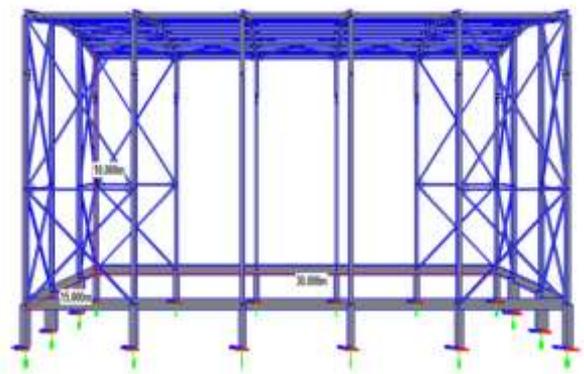
**Table-1 : Soil Elastic Constants**

Soil type	Modulus of Elasticity (kN/m <sup>2</sup> )	Unit Wt. ( $\gamma$ )	Poisson ratio ( $\mu$ )
Hard	65000	16	0.3
Medium	35000	16	0.4
Soft	15000	16	0.3

### 2.1 General data of building



**Fig.1:** Elevation



**Fig.2:** Side view

**Table -2:** Geometric and material properties of building

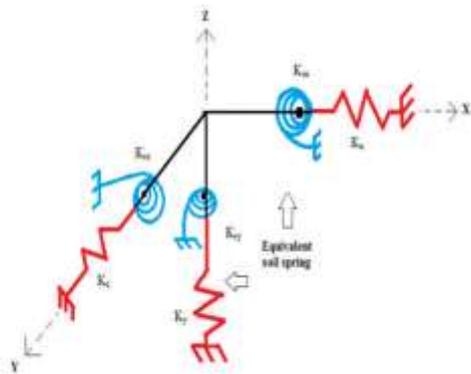
Structure	PEB and C.S.B
Spans	15m, 21m, 24m, 30m.
Column Height (m)	10m
Roof slope	1:10 and 1:5
Crane height (m)	8m
Bay spacing	6m
Purlin spacing	1.507m and 1.53m
Steel grade	345 Mpa and 250 Mpa
L/W	2

**Table -3:** Loads and combinations

1. Earthquake Load	IS 1893-2002
2. Dead Load	Selfweight + 0.1kN/m2(GI Sheet)
3. Live Load	0.75kN/m2
4. Colateral Load	0.05kN/m2
5. Wind Load	IS 875-part 3.
6. Crane Load	10 Tonne
7. DL +LL	IS 800-1984
8. DL + EQ	IS 800-1984
9. DL + CL	IS 800-1984
10. DL+LL+WL	IS 800-1984
11. DL + WL	IS 800-1984
12. DL + CnL	IS 800-1984
13. DL+LL+EQ	IS 800-1984
14. 0.75(DL+LL+EQ+CnL)	IS 800-1984
15. 0.75(DL+LL+WL+CnL)	IS 800-1984
16. 0.75(DL+WL+CnL)	IS 800-1984

**2.2 Winkler’s spring model**

Soil structure interaction is carried out by using Winkler’s approach [1] by considering equivalent springs with six degree of freedom (fig.3) which represents the soil medium. Each spring has specific stiffness which depends upon the properties of respective soil conditions. The stiffness is calculated by George Gazetas formulas[8] and shown in table 4



**Fig.3:** Equivalent spring stiffness.

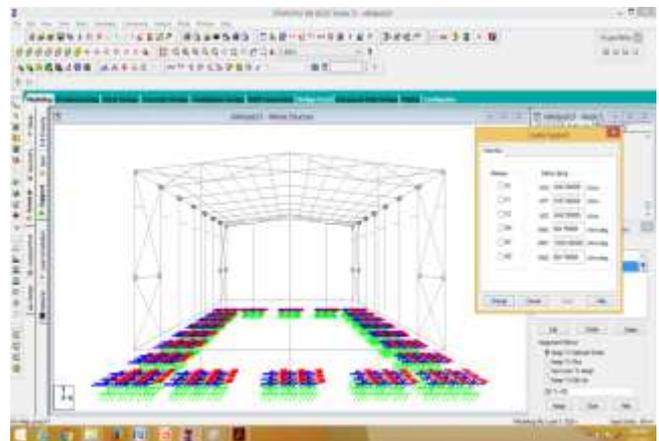
Where,  $K_x, K_y, K_z$  = Stiffness of equivalent soil springs along the translational DOF along X, Y and Z axis.  $K_{rx}, K_{ry}, K_{rz}$  = Stiffness of equivalent soil springs along the rotational DOF along X, Y and Z axis.

**Table -4:** Spring stiffness formulas (G Gazetas)

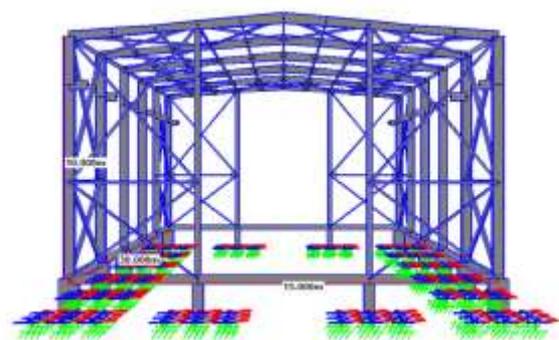
Degrees of freedom	Stiffness of equivalent soil spring
Horizontal (lateral)	$[2GL/(2-v)](2+2.50\chi^{0.85})$ with $\chi = A_b/4L^2$
Horizontal (longitudinal)	$[2GL/(2-v)](2+2.50\chi^{0.85})-[0.2/(0.75-v)]GL$ $[1-(B/L)]$ with $\chi = A_b/4L^2$
Vertical	$[2GL/(1-v)](0.73+1.54\chi^{0.75})$ with $\chi = A_b/4L^2$
Rocking(about longitudinal)	$[G/(1-v)]I_{bx}^{0.75}(L/B)^{0.25}[2.4+0.5(B/L)]$
Rocking (about lateral)	$[G/(1-v)]I_{by}^{0.75}(L/B)^{0.15}$
Torsion	$3.5G I_{bz}^{0.75}(B/L)^{0.4}(I_{bz}/B^4)^{0.2}$

Where,  $A_b$  area of the foundation considered,  $B$  and  $L$  half-width and half length of a foundation respectively.  $I_{bx}, I_{by}$ , and  $I_{bz}$  moment of inertia of foundation area with respect to longitudinal, vertical and lateral axes respectively.

Distribution of the stiffness of springs obtained for soil strata considered below foundation is distributed uniformly below foundation [9] as shown in fig.4,5,6,7,8.



**Fig.4:** Application of soil stiffness in STAAD Pro.



**Fig-5:** Distribution of springs in 15m P.E.B.

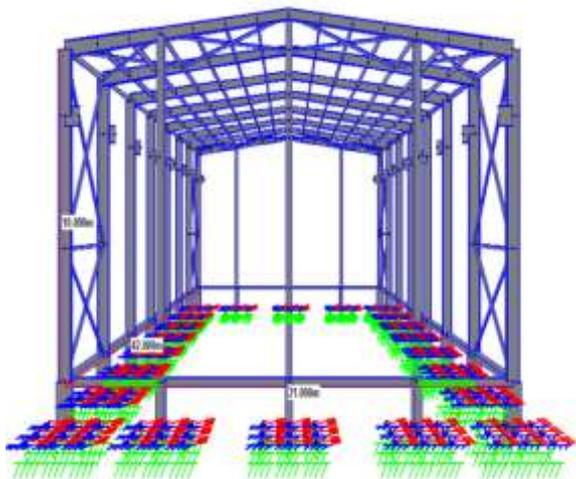


Fig.6: Distribution of springs in 21m P.E.B.

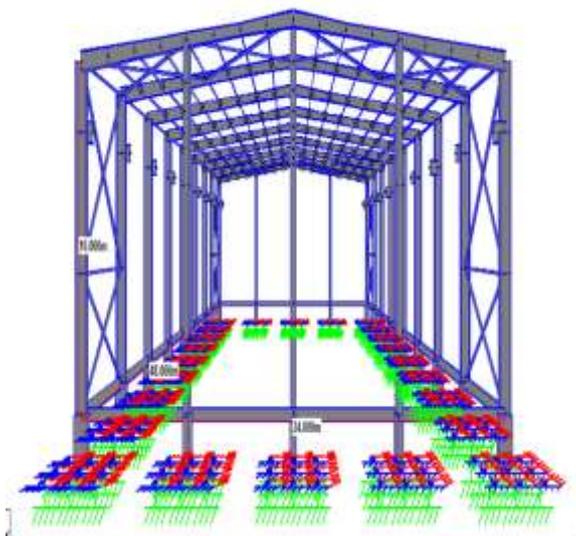


Fig.7: Distribution of springs in 24m P.E.B.

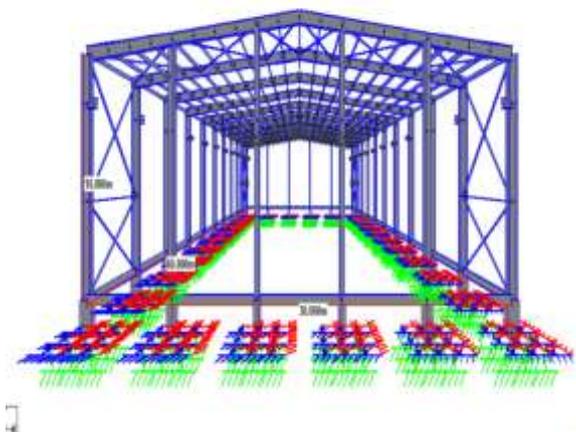


Fig.8: Distribution of springs in 30m P.E.B.

III. PARAMETRIC STUDY

The effect of different base condition on performance of P.E.B is studied considering soil structure interaction (SSI).

Effect of SSI on P.E.B is carried out considering the following parametric study.

3.1 Base shear

The variation of base shear for different soil conditions with respect to varying spans is represented as follow;

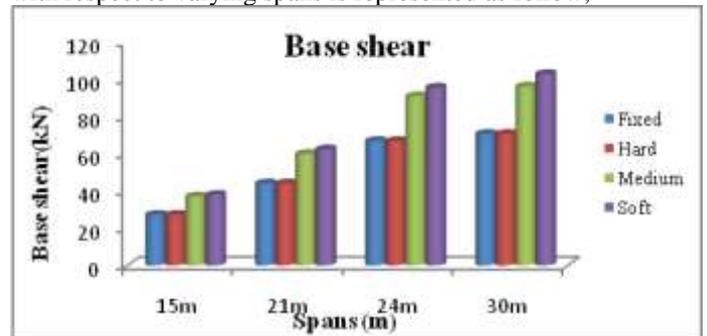


Chart .1: Base shear for different Soils.

3.2 Lateral deflection

The variation of lateral deflection for different types of soil conditions with respect to varying spans is represented as follow;

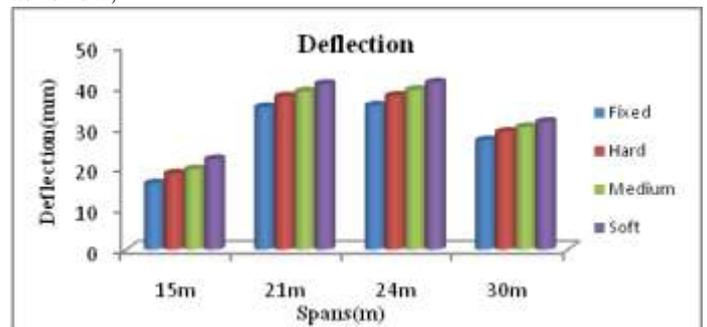


Chart .2: Lateral displacement for various soils.

3.3 Column moment

The variation of Column moment for different types of soil conditions with respect to varying spans is represented as follow;

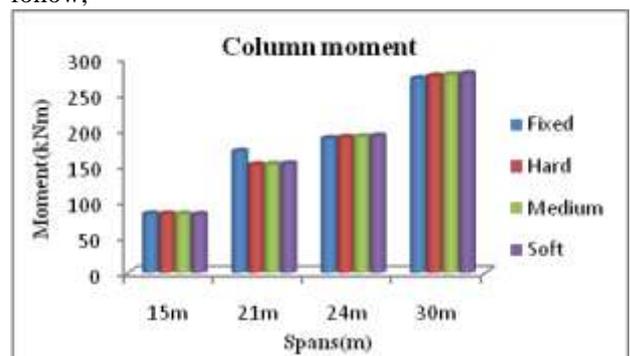


Chart .3: Column moment for different soil.

IV. CONCLUSIONS

1. Percentage increase in Base shear of 15,21,24 and 30 m span P.E.B for soft soil compared to fixed support are 39.14%, 41.63%, 43% and 45.5%
2. Percentage increase in lateral deflection of 15,21,24 and 30 m span P.E.B for soft soil

compared to fixed support are 36.44%, 16.11%, 16% and 17%

3. It is observed that column moment of 15,21m span decreases by 1%,11.5% and of 24,30m span increases by 2%, 2.5%
4. Column moment of structure Changes due to SSI effect. It changes with the flexibility of soil, column moment is main parameter while designing P.E.B structure. Thus evaluation of this parameter without considering SSI effect may cause severe error in design of P.E.B

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